KonstruX fully threaded screws

The powerful solution for construction and renovation

ETA CE
Europ. Techn. Bewertung
European Technical Assessment
ETA-11/0024

What can it be used for?

- For all timber-frame construction joints
- Timber engineering
- Carpentry
- Timber-frame construction
- Construction of timber elements
- Hall construction
- Renovation of ceilings

Properties

- Maximum load transmission
- High fire-resistance
- No thermal bridges

Advantages

- High extraction resistance
- Strong joints
- Maximisation of the load-bearing capacity
- A time- and cost-saving alternative
- Hidden connections
- No pre-drilling required according to approval / ETA (recommended from screw lengths ≥ 245 mm)

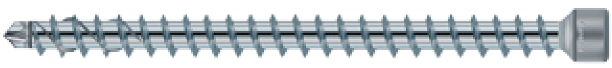
KonstruX ST cylinder head, galvanised

KonstruX ST countersunk head, galvanised

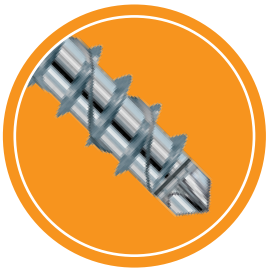
KonstruX countersunk head, galvanised

KonstruX ST fully threaded screw

Cylinder head, galvanised



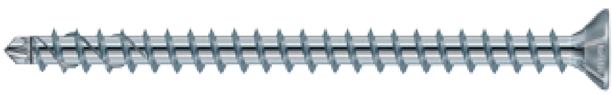
Benefits of new drill point
- Reduced screwing torque
- High extraction resistance



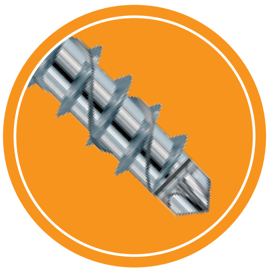
Art. no.	Dimensions [mm]	Drive	PU
904808	6,5 x 80	TX30	100
904809	6,5 x 100	TX30	100
904810	6,5 x 120	TX30	100
904811	6,5 x 140	TX30	100
904812	6,5 x 160	TX30	100
904813	6,5 x 195	TX30	100
904825	8,0 x 155	TX40	50
904826	8,0 x 195	TX40	50
904827	8,0 x 220	TX40	50
904828	8,0 x 245	TX40	50
904829	8,0 x 295	TX40	50
904830	8,0 x 330	TX40	50
904831	8,0 x 375	TX40	50
904832	8,0 x 400	TX40	50
944804	8,0 x 430	TX40	50
944805	8,0 x 480	TX40	50
904815	10,0 x 300	TX50	25
904816	10,0 x 330	TX50	25
904817	10,0 x 360	TX50	25
904818	10,0 x 400	TX50	25
904819	10,0 x 450	TX50	25
904820	10,0 x 500	TX50	25
904821	10,0 x 550	TX50	25
904822	10,0 x 600	TX50	25

KonstruX ST fully threaded screw

Countersunk head, galvanised



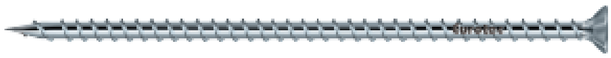
Benefits of new drill point
- Reduced screwing torque
- High extraction resistance



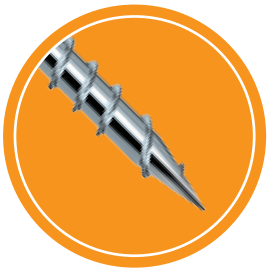
Art. no.	Dimensions [mm]	Drive	PU
904790	8,0 x 95	TX40	50
904791	8,0 x 125	TX40	50
904792	8,0 x 155	TX40	50
904793	8,0 x 195	TX40	50
904794	8,0 x 220	TX40	50
904795	8,0 x 245	TX40	50
904796	8,0 x 270	TX40	50
904797	8,0 x 295	TX40	50
904798	8,0 x 330	TX40	50
904799	8,0 x 375	TX40	50
904800	8,0 x 400	TX40	50
904801	8,0 x 430	TX40	50
904802	8,0 x 480	TX40	50
904770	10,0 x 125	TX50	25
904771	10,0 x 155	TX50	25
904772	10,0 x 195	TX50	25
904773	10,0 x 220	TX50	25
904774	10,0 x 245	TX50	25
904775	10,0 x 270	TX50	25
904776	10,0 x 300	TX50	25
904777	10,0 x 330	TX50	25
904778	10,0 x 360	TX50	25
904779	10,0 x 400	TX50	25
904780	10,0 x 450	TX50	25
904781	10,0 x 500	TX50	25
904782	10,0 x 550	TX50	25
904783	10,0 x 600	TX50	25

KonstruX fully threaded screw

Countersunk head, galvanised



Advantages AG screw tip
- Faster and easier screwing
- Reduced splitting effect



Art. no.	Dimensions [mm]	Drive	PU
905737	11,3 x 300	TX50 ●	20
905738	11,3 x 340	TX50 ●	20
905739	11,3 x 380	TX50 ●	20
905740	11,3 x 420	TX50 ●	20
905741	11,3 x 460	TX50 ●	20
905742	11,3 x 500	TX50 ●	20
905743	11,3 x 540	TX50 ●	20
905744	11,3 x 580	TX50 ●	20
905745	11,3 x 620	TX50 ●	20
905746	11,3 x 660	TX50 ●	20
905747	11,3 x 700	TX50 ●	20
905748	11,3 x 750	TX50 ●	20
905749	11,3 x 800	TX50 ●	20
904750	11,3 x 900	TX50 ●	20
904751	11,3 x 1000	TX50 ●	20

KonstruX

NEW Available in Stainless Steel A4

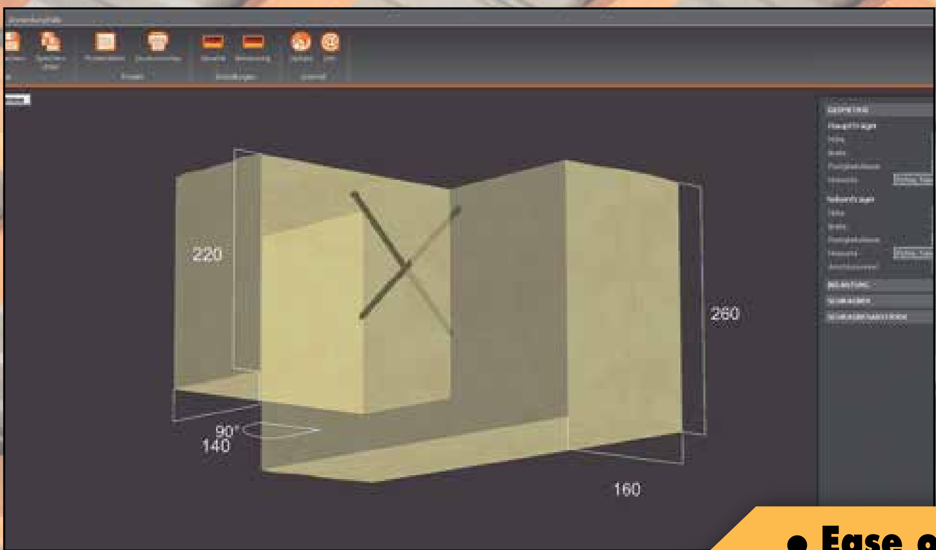
Countersunk-head screw, A4

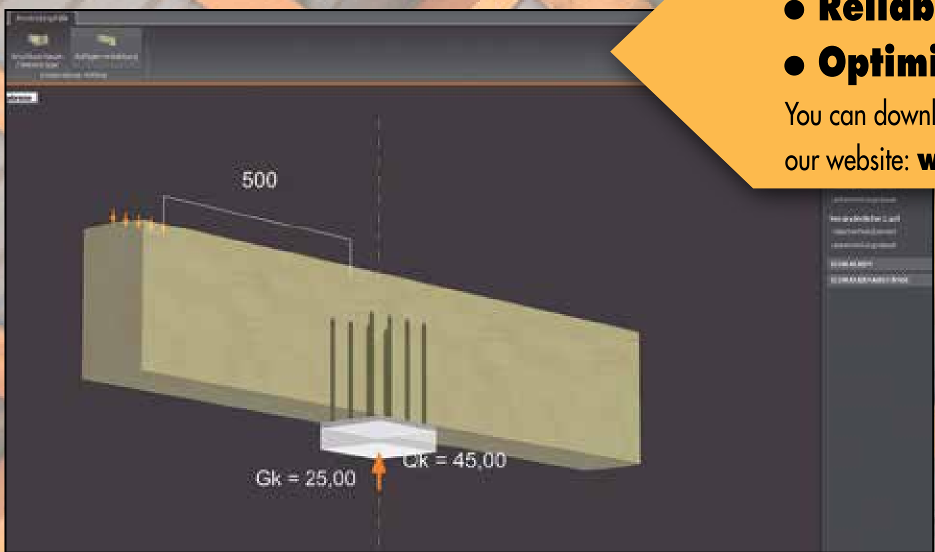
NEW to our product range

Art. no.	Dimensions [mm]	Drive	PU
905750	10,0 x 160	TX50 ●	25
905751	10,0 x 200	TX50 ●	25
905752	10,0 x 220	TX50 ●	25
905753	10,0 x 240	TX50 ●	25
905754	10,0 x 260	TX50 ●	25
905755	10,0 x 280	TX50 ●	25
905756	10,0 x 300	TX50 ●	25
905757	10,0 x 350	TX50 ●	25
905758	10,0 x 400	TX50 ●	25

ECS calculation program for KonstruX

This user-friendly software allows pre-calculation of main/secondary beam connections, joist doubling and reinforcements of supports. Verifiable calculation aid in accordance with EN 1995 (Eurocode 5) and DIN 1052.

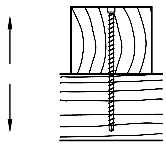
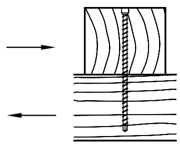
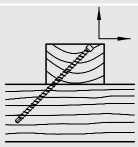
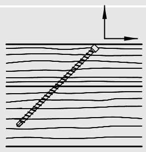
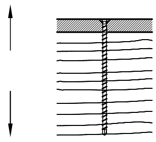
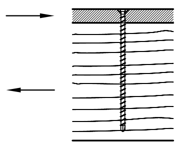
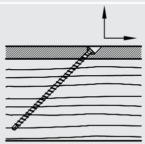
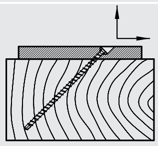
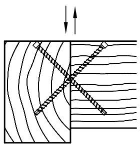
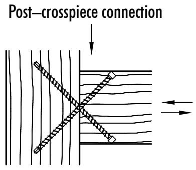
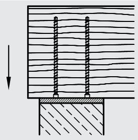
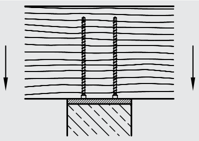
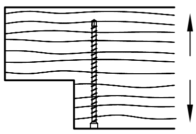
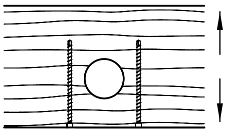
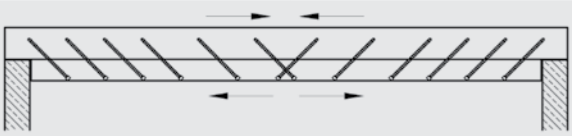
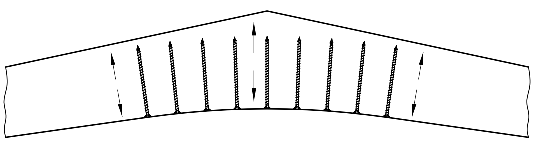




- **Ease of use**
- **Reliable planning**
- **Optimisation**

You can download the ECS software for free from our website: **www.eurotec.team/en**

The fast and secure timber-joint system
KonstruX cylinder-head /countersunk-head screws

Example applications		Cylinder head			Countersunk head		
		Ø 6,5 [mm]	Ø 8,0 [mm]	Ø 10,0 [mm]	Ø 8,0 [mm]	Ø 10,0 [mm]	Ø 11,3 [mm]
Timber–timber tensile loading 	Timber–timber shearing 	×	×	×	×	×	×
Timber–timber under tension at 45° 	Timber–timber under tension at 45° 	×	×	×	×	×	×
Steel–timber tensile loading 	Steel–timber shearing 	—	—	—	×	×	×
Timber–timber under tension at 45° 	Timber–timber under tension at 45° 	—	—	—	×	×	×
Main–secondary beam connection 	Post–crosspiece connection 	×	×	×	×	×	—
Support reinforcement 	Support reinforcement 	×	×	×	×	×	×
Transverse-shear reinforcement at notch 	Transverse-shear reinforcement at opening 	×	×	×	×	×	×
Joist doubling 		—	×	×	×	×	×
Transverse-shear reinforcement of building trusses 		—	—	×	—	×	×

Eurotec®

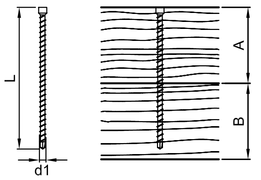
Europ. Techn. Bewertung
European Technical Assessment
ETA-11/0024

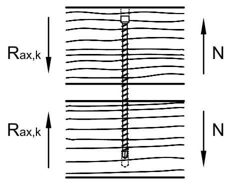
KonstruX fully threaded screw

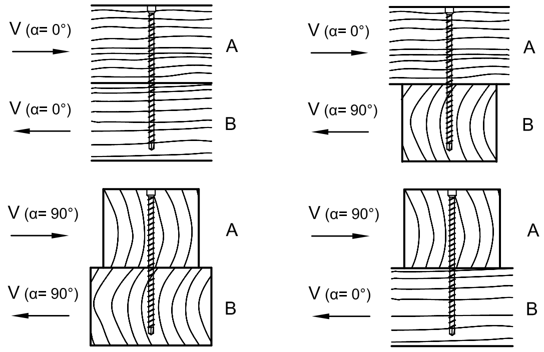
Technical information

KonstruX ST with cylinder head and new drill point 6,5 to 10,0 mm: timber / timber joints

Dimensions	Extraction resistance	Shearing







Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ acc. to ETA-11/0024

Characteristic value of the joint's loadbearing capacity R_k acc. to ETA-11/0024

d1 x L [mm]	A [mm]	B [mm]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]
				α= 0°	α= 90°	α_A= 0° α_B= 90°	α_A= 90° α_B= 0°
6,5 x 120	60	80	4,75	3,93	3,47	3,93	3,47
6,5 x 140	80	80	4,75	3,93	3,47	3,47	3,93
6,5 x 160	80	100	6,33	4,32	3,86	4,32	3,86
6,5 x 195	100	100	7,52	4,62	4,16	4,16	4,62
8,0 x 155	80	80	7,11	5,67	4,99	4,99	5,67
8,0 x 195	100	100	9,01	6,15	5,46	5,46	6,15
8,0 x 220	120	120	9,48	6,27	5,58	5,58	6,27
8,0 x 245	120	140	11,38	6,74	6,06	6,74	6,06
8,0 x 295	140	160	13,28	7,21	6,42	7,21	6,42
8,0 x 330	160	180	15,17	7,69	6,42	7,69	6,42
8,0 x 375	180	200	17,07	7,79	6,42	7,79	6,42
8,0 x 400	200	220	18,97	7,79	6,42	7,79	6,42
8,0 x 430	220	220	19,92	7,79	6,42	6,42	7,79
8,0 x 480	240	260	22,76	7,79	6,42	7,79	6,42
10,0 x 300	160	160	16,15	9,48	8,48	8,48	9,48
10,0 x 330	160	180	18,46	10,06	8,90	10,06	8,90
10,0 x 360	180	200	20,76	10,64	8,90	10,64	8,90
10,0 x 400	200	220	23,07	10,89	8,90	10,89	8,90
10,0 x 450	220	240	25,38	10,89	8,90	10,89	8,90
10,0 x 500	240	280	27,68	10,89	8,90	10,89	8,90
10,0 x 550	260	300	29,99	10,89	8,90	10,89	8,90
10,0 x 600	300	320	33,00	10,89	8,90	10,89	8,90

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: R_d= $R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

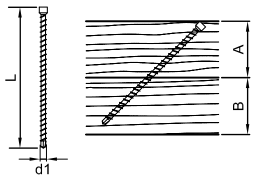
Example:

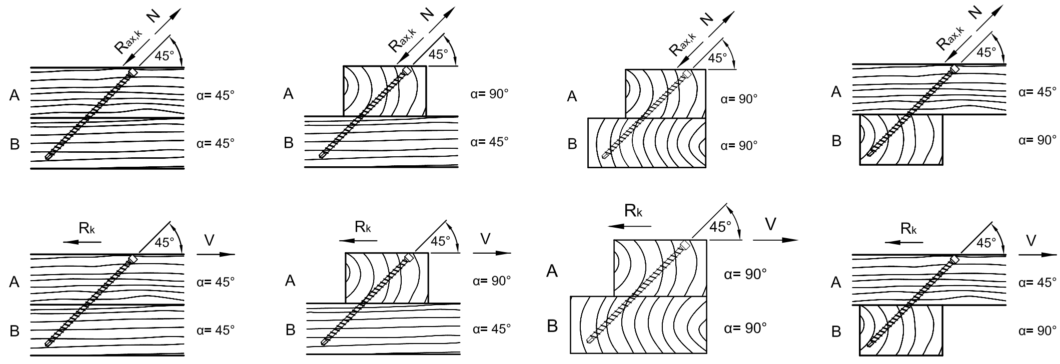
Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= 7,20 kN.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min R_k= $R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min R_k= $R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KonstruX ST with cylinder head and new drill point 6,5 to 10,0 mm: timber / timber joints

Dimensions	Tension connection





Characteristic value of the joint's load-bearing capacity $R_{ax,k}$ bzw. R_k acc. to ETA-11/0024

d1 x L [mm]	A [mm]	B [mm]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]
			α= 45°		α_A= 90° α_B= 45°		α_A= 90° α_B= 90°		α_A= 45° α_B= 90°	
6,5 x 160	60	80	5,95	4,21	5,95	4,21	5,95	4,21	5,95	4,21
6,5 x 195	80	80	6,48	4,58	6,48	4,58	6,48	4,58	6,48	4,58
8,0 x 155	60	60	6,65	4,70	6,65	4,70	6,65	4,70	6,65	4,70
8,0 x 195	80	80	7,76	5,49	7,76	5,49	7,76	5,49	7,76	5,49
8,0 x 220	80	100	10,13	7,17	10,13	7,17	10,13	7,17	10,13	7,17
8,0 x 245	100	100	9,82	6,95	9,82	6,95	9,82	6,95	9,82	6,95
8,0 x 295	120	100	11,88	8,40	11,88	8,40	11,88	8,40	11,88	8,40
8,0 x 330	120	140	15,20	10,75	15,20	10,75	15,20	10,75	15,20	10,75
8,0 x 375	140	140	16,79	11,87	16,79	11,87	16,79	11,87	16,79	11,87
8,0 x 400	160	140	16,48	11,65	16,48	11,65	16,48	11,65	16,48	11,65
8,0 x 430	160	160	19,32	13,66	19,32	13,66	19,32	13,66	19,32	13,66
8,0 x 480	180	180	21,38	15,12	21,38	15,12	21,38	15,12	21,38	15,12
10,0 x 300	120	120	15,03	10,63	15,03	10,63	15,03	10,63	15,03	10,63
10,0 x 330	120	140	18,49	13,07	18,49	13,07	18,49	13,07	18,49	13,07
10,0 x 360	140	140	18,69	13,21	18,69	13,21	18,69	13,21	18,69	13,21
10,0 x 400	160	140	20,04	14,17	20,04	14,17	20,04	14,17	20,04	14,17
10,0 x 450	160	180	25,81	18,25	25,81	18,25	25,81	18,25	25,81	18,25
10,0 x 500	180	200	28,31	20,02	28,31	20,02	28,31	20,02	28,31	20,02
10,0 x 550	200	200	30,82	21,79	30,82	21,79	30,82	21,79	30,82	21,79
10,0 x 600	220	220	33,00	23,33	33,00	23,33	33,00	23,33	33,00	23,33

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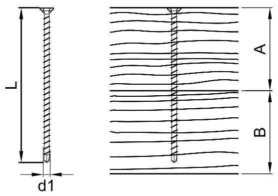
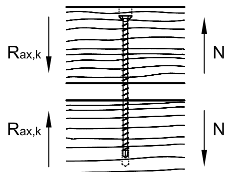
i.e. the characteristic minimum value is calculated based on: min $R_k = R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.

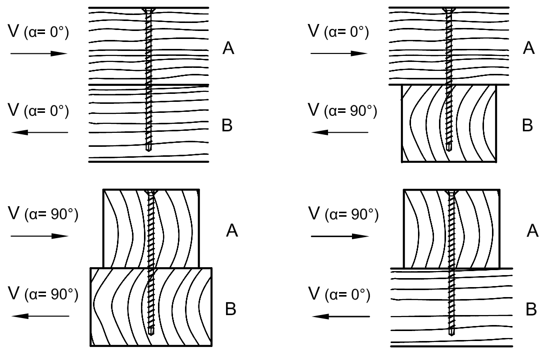
Please note: These are planning aids. Projects must only be calculated by authorised persons.

KonstruX ST with countersunk head and new drill point 8,0 and 10,0 mm: timber/timber joints

Dimensions	Extraction resistance	Shearing







Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ acc. to ETA-11/0024

Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ acc. to ETA-11/0024

d1 x L [mm]	A [mm]	B [mm]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]
				α= 0°	α= 90°	α_A= 0°, α_B= 90°	α_A= 90°, α_B= 0°
8,0 x 95	40	60	3,08	4,61	3,57	4,61	3,57
8,0 x 125	60	80	4,61	5,05	4,37	5,05	4,37
8,0 x 155	80	80	7,11	5,67	4,99	4,99	5,67
8,0 x 195	100	100	9,01	6,15	5,46	5,46	6,15
8,0 x 220	120	120	9,48	6,27	5,58	5,58	6,27
8,0 x 245	120	140	11,38	6,74	6,06	6,74	6,06
8,0 x 270	140	140	12,33	6,98	6,29	6,29	6,98
8,0 x 295	140	160	13,28	7,21	6,42	7,21	6,42
8,0 x 330	160	180	15,17	7,69	6,42	7,69	6,42
8,0 x 375	180	200	17,07	7,79	6,42	7,79	6,42
8,0 x 400	200	220	18,97	7,79	6,42	7,79	6,42
8,0 x 430	220	220	19,92	7,79	6,42	6,42	7,79
8,0 x 480	240	260	22,76	7,79	6,42	7,79	6,42
10,0 x 125	60	80	6,92	7,18	6,18	7,18	6,18
10,0 x 155	80	80	8,65	7,61	6,61	6,61	7,61
10,0 x 195	100	100	10,96	8,19	7,19	7,19	8,19
10,0 x 220	120	120	11,53	8,33	7,33	7,33	8,33
10,0 x 245	120	140	13,84	8,91	7,91	8,91	7,91
10,0 x 270	140	140	14,99	9,20	8,20	8,20	9,20
10,0 x 300	160	160	16,15	9,48	8,48	8,48	9,48
10,0 x 330	160	180	18,46	10,06	8,90	10,06	8,90
10,0 x 360	180	200	20,76	10,64	8,90	10,64	8,90
10,0 x 400	200	220	23,07	10,89	8,90	10,89	8,90
10,0 x 450	220	240	25,38	10,89	8,90	10,89	8,90
10,0 x 500	240	280	27,68	10,89	8,90	10,89	8,90
10,0 x 550	260	300	29,99	10,89	8,90	10,89	8,90
10,0 x 600	300	320	33,00	10,89	8,90	10,89	8,90

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: $R_d = R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

Example:

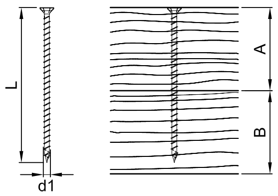
Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= 7,20 kN.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min $R_k = R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min $R_k = R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.

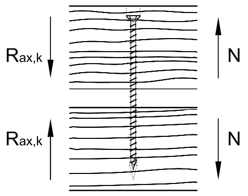
Please note: These are planning aids. Projects must only be calculated by authorised persons.

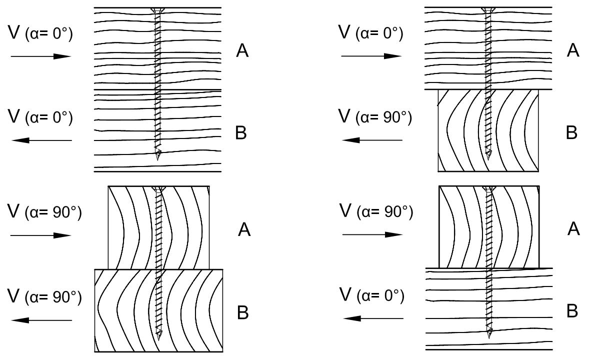
Eurotec®

KonstruX with countersunk head and drill point or AG tip 11,3 mm: timber / timber connection

Dimensions	Extraction resistance	Shearing







Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ acc. to ETA-11/0024

Characteristic value of the joint's loadbearing capacity R_k acc. to ETA-11/0024

d1 x L [mm]	A [mm]	B [mm]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]
				α= 0°	α= 90°	α_A= 0° / α_B= 90°	α_A= 90° / α_B= 0°
11,3 x 300	160	160	18,25	12,17	10,73	10,73	12,17
11,3 x 340	180	180	20,85	12,82	11,38	11,38	12,82
11,3 x 380	200	200	23,46	13,47	12,03	12,03	13,47
11,3 x 420	220	220	26,07	14,12	12,34	12,34	14,12
11,3 x 460	240	240	26,67	14,77	12,34	12,34	14,77
11,3 x 500	260	260	31,28	15,21	12,34	12,34	15,21
11,3 x 540	280	280	33,89	15,21	12,34	12,34	15,21
11,3 x 580	300	300	36,49	15,21	12,34	12,34	15,21
11,3 x 620	320	320	39,10	15,21	12,34	12,34	15,21
11,3 x 660	340	340	41,71	15,21	12,34	12,34	15,21
11,3 x 700	360	360	44,32	15,21	12,34	12,34	15,21
11,3 x 750	380	380	48,23	15,21	12,34	12,34	15,21
11,3 x 800	400	420	50,00	15,21	12,34	15,21	12,34
11,3 x 900	460	460	50,00	15,21	12,34	12,34	15,21
11,3 x 1000	500	520	50,00	15,21	12,34	15,21	12,34

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: $R_d = R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

Example:

Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
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i.e. the characteristic minimum value is calculated based on: min $R_k = R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.

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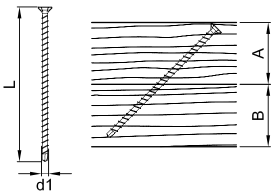
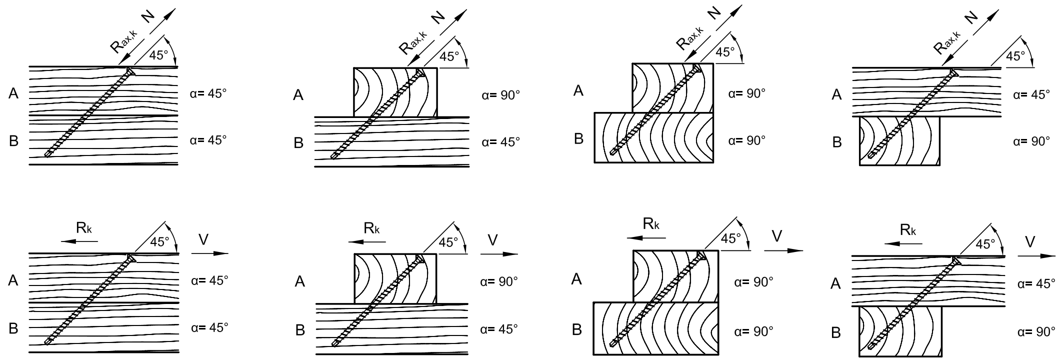
KonstruX ST with countersunk head and new drill point 8,0 and 10,0 mm: timber / timber joints

Dimensions	Tension connection





Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ bzw. R_k acc. to ETA-11/0024

d1 x L [mm]	A [mm]	B [mm]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]
			α= 45°		α_A= 90° α_B= 45°		α_A= 90° α_B= 90°		α_A= 45° α_B= 90°	
8,0 x 155	60	60	6,65	4,70	6,65	4,70	6,65	4,70	6,65	4,70
8,0 x 195	80	80	7,76	5,49	7,76	5,49	7,76	5,49	7,76	5,49
8,0 x 220	80	100	10,13	7,17	10,13	7,17	10,13	7,17	10,13	7,17
8,0 x 245	100	100	9,82	6,95	9,82	6,95	9,82	6,95	9,82	6,95
8,0 x 270	100	120	12,19	8,62	12,19	8,62	12,19	8,62	12,19	8,62
8,0 x 295	120	100	11,88	8,40	11,88	8,40	11,88	8,40	11,88	8,40
8,0 x 330	120	140	15,20	10,75	15,20	10,75	15,20	10,75	15,20	10,75
8,0 x 375	140	140	16,79	11,87	16,79	11,87	16,79	11,87	16,79	11,87
8,0 x 400	160	140	16,48	11,65	16,48	11,65	16,48	11,65	16,48	11,65
8,0 x 430	160	160	19,32	13,66	19,32	13,66	19,32	13,66	19,32	13,66
8,0 x 480	180	180	21,38	15,12	21,38	15,12	21,38	15,12	21,38	15,12
10,0 x 220	80	100	12,33	8,72	12,33	8,72	12,33	8,72	12,33	8,72
10,0 x 245	100	100	11,95	8,45	11,95	8,45	11,95	8,45	11,95	8,45
10,0 x 270	100	120	14,83	10,49	14,83	10,49	14,83	10,49	14,83	10,49
10,0 x 300	120	120	15,03	10,63	15,03	10,63	15,03	10,63	15,03	10,63
10,0 x 330	120	140	18,49	13,07	18,49	13,07	18,49	13,07	18,49	13,07
10,0 x 360	140	140	18,69	13,21	18,69	13,21	18,69	13,21	18,69	13,21
10,0 x 400	160	140	20,04	14,17	20,04	14,17	20,04	14,17	20,04	14,17
10,0 x 450	160	180	25,81	18,25	25,81	18,25	25,81	18,25	25,81	18,25
10,0 x 500	180	200	28,31	20,02	28,31	20,02	28,31	20,02	28,31	20,02
10,0 x 550	200	200	30,82	21,79	30,82	21,79	30,82	21,79	30,82	21,79
10,0 x 600	220	220	33,00	23,33	33,00	23,33	33,00	23,33	33,00	23,33

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
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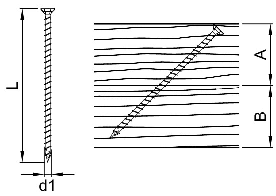
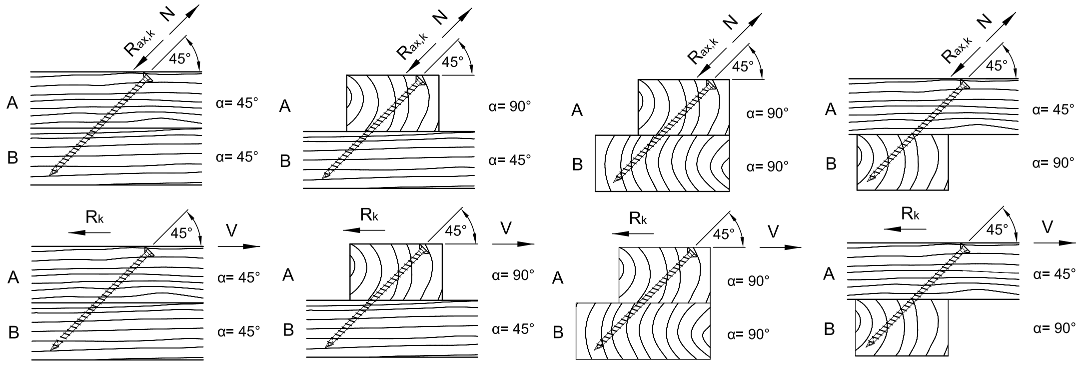
KonstruX with countersunk head and drill point or AG tip 11,3 mm: timber / timber connection

Dimensions	Tension connection





Characteristic value of the joint's load-bearing capacity $R_{ax,k}$ or R_k acc. to ETA-11/0024

d1 x L [mm]	A [mm]	B [mm]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]
			α= 45°		$α_A$= 90° $α_B$= 45°		$α_A$= 90° $α_B$= 90°		$α_A$= 45° $α_B$= 90°	
11,3 x 300	120	120	16,98	12,01	16,98	12,01	16,98	12,01	16,98	12,01
11,3 x 340	140	120	18,51	13,09	18,51	13,09	18,51	13,09	18,51	13,09
11,3 x 380	140	140	23,72	16,77	23,72	16,77	23,72	16,77	23,72	16,77
11,3 x 420	160	160	25,25	17,85	25,25	17,85	25,25	17,85	25,25	17,85
11,3 x 460	180	160	26,78	18,93	26,78	18,93	26,78	18,93	26,78	18,93
11,3 x 500	180	200	31,99	22,62	31,99	22,62	31,99	22,62	31,99	22,62
11,3 x 540	200	200	33,52	23,70	33,52	23,70	33,52	23,70	33,52	23,70
11,3 x 580	220	220	35,04	24,78	35,04	24,78	35,04	24,78	35,04	24,78
11,3 x 620	220	240	40,26	28,47	40,26	28,47	40,26	28,47	40,26	28,47
11,3 x 660	240	240	41,79	29,55	41,79	29,55	41,79	29,55	41,79	29,55
11,3 x 700	260	260	43,31	30,63	43,31	30,63	43,31	30,63	43,31	30,63
11,3 x 750	280	280	46,14	32,63	46,14	32,63	46,14	32,63	46,14	32,63
11,3 x 800	300	280	48,97	34,63	48,97	34,63	48,97	34,63	48,97	34,63
11,3 x 900	320	340	50,00	35,36	50,00	35,36	50,00	35,36	50,00	35,36
11,3 x 1000	360	360	50,00	35,36	50,00	35,36	50,00	35,36	50,00	35,36

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
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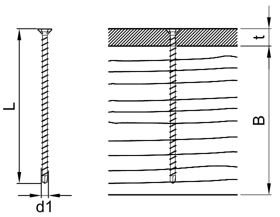
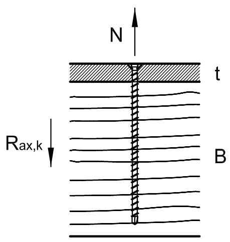
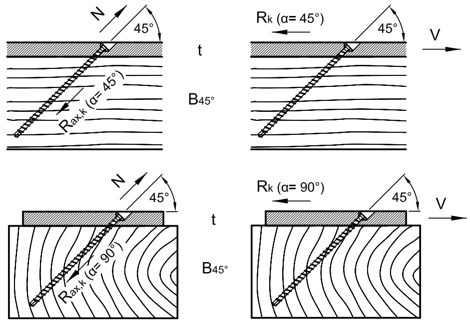
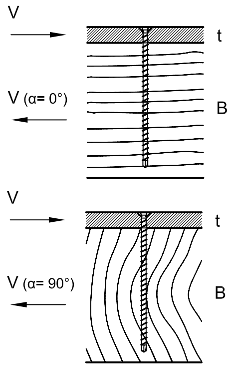
KonstruX ST with countersunk head and new drill point 8,0 and 10,0 mm: steel / timber joints









Dimensions				Extraction resistance	Tension connection				Shearing	
				Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ acc. to ETA-11/0024	Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ bzw. R_k acc. to ETA-11/0024				Characteristic value of the joint's loadbearing capacity R_k acc. toETA-11/0024	
d1 x L [mm]	t [mm]	B [mm]	$B_{45°}$ [mm]	$R_{ax,k}$ a) - [kN]	$R_{ax,k}$ a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]
					α= 45°	α=90°	α= 45°	α= 90°	α= 0°	α= 90°
8,0 x 95	15	100	80	7,59	7,00	7,00	4,95	4,95	6,18	5,22
8,0 x 125	15	120	100	10,43	9,84	9,84	6,96	6,96	6,18	5,22
8,0 x 155	15	160	120	13,28	12,69	12,69	8,97	8,97	6,18	5,22
8,0 x 195	15	200	140	17,07	16,48	16,48	11,65	11,65	6,18	5,22
8,0 x 220	15	220	160	19,44	18,85	18,85	13,33	13,33	6,18	5,22
8,0 x 245	15	240	180	21,81	21,22	21,22	15,01	15,01	6,18	5,22
8,0 x 270	15	280	200	24,18	23,59	23,59	16,68	16,68	6,18	5,22
8,0 x 295	15	300	220	25,00	25,00	25,00	17,68	17,68	6,18	5,22
8,0 x 330	15	340	240	25,00	25,00	25,00	17,68	17,68	6,18	5,22
8,0 x 375	15	380	280	25,00	25,00	25,00	17,68	17,68	6,18	5,22
8,0 x 400	15	400	280	25,00	25,00	25,00	17,68	17,68	6,18	5,22
8,0 x 430	15	440	300	25,00	25,00	25,00	17,68	17,68	6,18	5,22
8,0 x 480	15	480	340	25,00	25,00	25,00	17,68	17,68	6,18	5,22
10,0 x 125	15	120	100	12,69	11,97	11,97	8,46	8,46	8,72	7,30
10,0 x 155	15	160	120	16,15	15,43	15,43	10,91	10,91	8,72	7,30
10,0 x 195	15	200	140	20,76	20,05	20,05	14,17	14,17	8,72	7,30
10,0 x 220	15	220	160	23,65	22,93	22,93	16,21	16,21	8,72	7,30
10,0 x 245	15	240	180	26,53	25,81	25,81	18,25	18,25	8,72	7,30
10,0 x 270	15	280	200	29,41	28,70	28,70	20,29	20,29	8,72	7,30
10,0 x 300	15	300	220	32,87	32,16	32,16	22,74	22,74	8,72	7,30
10,0 x 330	15	340	240	33,00	33,00	33,00	23,33	23,33	8,72	7,30
10,0 x 360	15	360	260	33,00	33,00	33,00	23,33	23,33	8,72	7,30
10,0 x 400	15	400	280	33,00	33,00	33,00	23,33	23,33	8,72	7,30
10,0 x 450	15	460	320	33,00	33,00	33,00	23,33	23,33	8,72	7,30
10,0 x 500	15	500	360	33,00	33,00	33,00	23,33	23,33	8,72	7,30
10,0 x 550	15	560	400	33,00	33,00	33,00	23,33	23,33	8,72	7,30
10,0 x 600	15	600	420	33,00	33,00	33,00	23,33	23,33	8,72	7,30

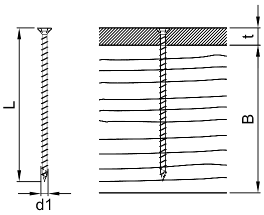
Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: $R_d = R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

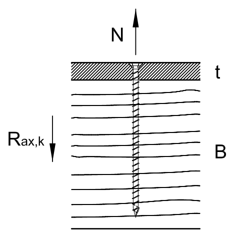
Example:

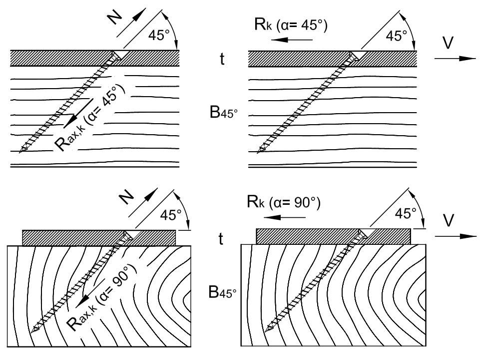
Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= 7,20 kN.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min $R_k = R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min $R_k = R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.

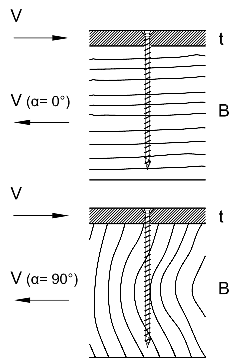
Please note: These are planning aids. Projects must only be calculated by authorised persons.

KonstruX with countersunk head and drill point or AG tip
11,3 mm: steel / timber connection









Dimensions				Extraction resistance	Tension connection				Shearing	
				Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ acc. to ETA-11/0024	Characteristic value of the joint's loadbearing capacity $R_{ax,k}$ bzw. R_k acc. to ETA-11/0024				Characteristic value of the joint's loadbearing capacity R_k acc. to ETA-11/0024	
d1 x L [mm]	t [mm]	B [mm]	$B_{45°}$ [mm]	$R_{ax,k}$ a) - [kN]	$R_{ax,k}$ a) - [kN]	$R_{ax,k}$ a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]	R_k a) - [kN]
					α= 45°	α=90°	α= 45°	α= 90°	α= 0°	α= 90°
11,3 x 300	20	300	220	36,49	35,42	35,42	25,04	25,04	11,79	9,76
11,3 x 340	20	340	240	41,71	40,63	40,63	28,73	28,73	11,79	9,76
11,3 x 380	20	380	260	46,92	45,84	45,84	32,42	32,42	11,79	9,76
11,3 x 420	20	420	300	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 460	20	460	320	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 500	20	500	360	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 540	20	540	380	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 580	20	580	420	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 620	20	620	440	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 660	20	660	460	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 700	20	700	500	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 750	20	740	540	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 800	20	800	560	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 900	20	900	640	50,00	50,00	50,00	35,36	35,36	11,79	9,76
11,3 x 1000	20	1000	700	50,00	50,00	50,00	35,36	35,36	11,79	9,76

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: $R_d = R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

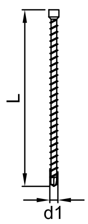
Example:

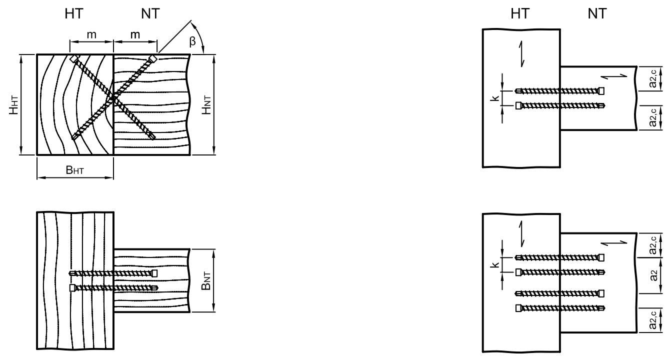
Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= 7,20 kN.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min $R_k = R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min $R_k = R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KonstruX ST with cylinder head and new drill point 6,5 mm: main / secondary beam joints

Dimensions	Main / secondary beam connection





a_2= min. 33 mm, $a_{2,c}$= min. 20 mm, k= min. 10 mm

Characteristic value of the joint's loadbearing capacity R_k acc. to ETA-11/0024

d1 x L [mm]	min. B_{NT} [mm]	min. H_{NT} [mm]	min. B_{HT} [mm]	min. H_{HT} [mm]	m [mm]	β °	$R_{v,k}$ a) b) - [kN]	Pair (n)
6,5 x 195	60	160	80	160	69	45	10,91	1
	100						20,36	2
	120						29,33	3
	160						38,00	4

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: $R_d= R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

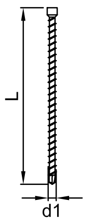
Example:

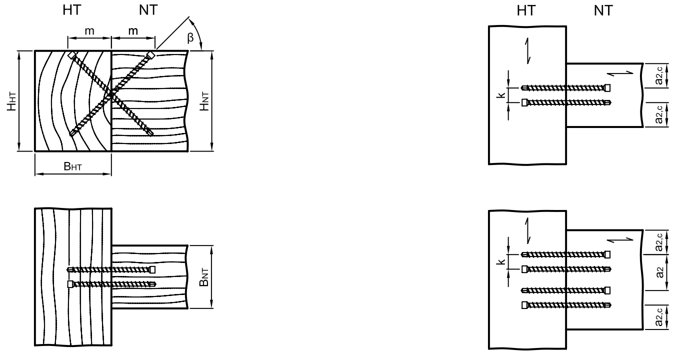
Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= <u>7,20 kN.</u>
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min $R_k= R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min $R_k= R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= <u>10,40 kN</u> → comparison with table values.
b) estimated with an efficient quantity of pairs of screws: $n^{0,9}$.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KonstruX ST with cylinder head and new drill point 8,0 mm: main / secondary beam joints

Dimensions	Main / secondary beam connection





a_2= min. 40 mm, $a_{2,c}$= min. 24 mm, k= min. 12 mm

Characteristic value of the joint's loadbearing capacity R_k acc. to ETA-11/0024

d1 x L [mm]	min. B_{NT} [mm]	min. H_{NT} [mm]	min. B_{HT} [mm]	min. H_{HT} [mm]	m [mm]	β °	$R_{v,k}$ a) b) - [kN]	Pair (n)
8,0 x 245	80	200	100	200	87	45	16,43	1
	100						30,66	2
	140						44,16	3
	180						57,21	4
8,0 x 295	80	220	120	220	104	45	19,78	1
	100						36,92	2
	140						53,17	3
	180						68,89	4
8,0 x 330	80	260	140	260	117	45	22,13	1
	100						41,30	2
	140						59,48	3
	180						77,06	4
8,0 x 375	80	280	160	280	133	45	25,15	1
	100						46,93	2
	140						67,59	3
	180						87,57	4
8,0 x 400	80	300	160	300	141	45	26,82	1
	100						50,06	2
	140						72,10	3
	180						93,41	4
8,0 x 430	80	320	180	320	152	45	28,84	1
	100						53,81	2
	140						77,51	3
	180						100,41	4
8,0 x 480	80	360	180	360	170	45	32,19	1
	100						60,07	2
	140						86,52	3
	180						112,09	4

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: R_d= $R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

Example:

Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= 7,20 kN.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min R_k= $R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min R_k= $R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= 10,40 kN → comparison with table values.
b) estimated with an efficient quantity of pairs of screws: $n^{0,9}$.

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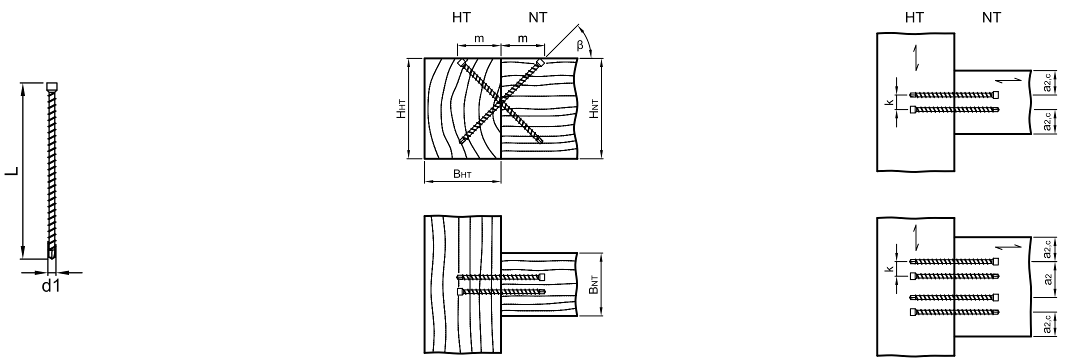
KonstruX ST with cylinder head and new drill point 10,0 mm: main / secondary beam joints

Dimensions	Main / secondary beam connection



a_2= min. 50 mm, $a_{2,c}$= min. 30 mm, k= min. 15 mm

Characteristic value of the joint's loadbearing capacity R_k acc. to ETA-11/0024

d1 x L [mm]	min. B_{NT} [mm]	min. H_{NT} [mm]	min. B_{HT} [mm]	min. H_{HT} [mm]	m [mm]	β °	$R_{v,k}$ $^{a) b)}$ - [kN]	Pair (n)
10,0 x 300	80	240	120	240	106	45	24,47	1
	140						45,66	2
	180						65,77	3
	240						85,20	4
10,0 x 330	80	260	140	260	117	45	26,92	1
	140						50,23	2
	180						72,34	3
	240						93,72	4
10,0 x 360	80	280	140	280	127	45	29,36	1
	140						54,79	2
	180						78,92	3
	240						102,24	4
10,0 x 400	80	300	160	300	141	45	32,62	1
	140						60,88	2
	180						87,69	3
	240						113,60	4
10,0 x 450	80	340	180	340	159	45	36,70	1
	140						68,49	2
	180						98,65	3
	240						127,80	4
10,0 x 500	80	380	200	380	177	45	40,78	1
	140						76,10	2
	180						109,61	3
	240						142,01	4
10,0 x 550	80	400	220	400	194	45	44,86	1
	140						83,71	2
	180						120,57	3
	240						156,21	4
10,0 x 600	80	440	240	440	212	45	46,67	1
	140						87,09	2
	180						125,44	3
	240						162,51	4

Calculation according to ETA-11/0024. Wood density ρ_k= 380 kg/m³. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
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a) The characteristic values of the load-bearing capacity R_k cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity R_k should be reduced to dimensioning values R_d with regard to the usage class and class of the load duration: $R_d= R_k \cdot k_{mod} / \gamma_M$. The dimensioning values of the load-bearing capacity R_d should be contrasted with the dimensioning values of the loads ($R_d \geq E_d$).

Example:

Characteristic value for constant load (dead weight) G_k= 2,00 kN and variable load (e. g. snow load) Q_k= 3,00 kN. k_{mod}= 0,9. γ_M= 1,3.
→ Dimensioning value of the load E_d= 2,00 · 1,35 + 3,00 · 1,5= <u>7,20 kN.</u>
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_d \geq E_d$. → min $R_k= R_d \cdot \gamma_M / k_{mod}$
i.e. the characteristic minimum value is calculated based on: min $R_k= R_d \cdot \gamma_M / k_{mod}$ → R_k= 7,20 kN · 1,3/0,9= <u>10,40 kN</u> → comparison with table values.
b) estimated with an efficient quantity of pairs of screws: $n^{0,9}$.

Please note: These are planning aids. Projects must only be calculated by authorised persons.